

Presented by

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## **AAPA Facilities Engineering Seminar**

**Jacksonville, Florida**

**11-13 January 2006**

- **Container Gantry**
- **Dry Bulk Gantry**
- **Mobile**





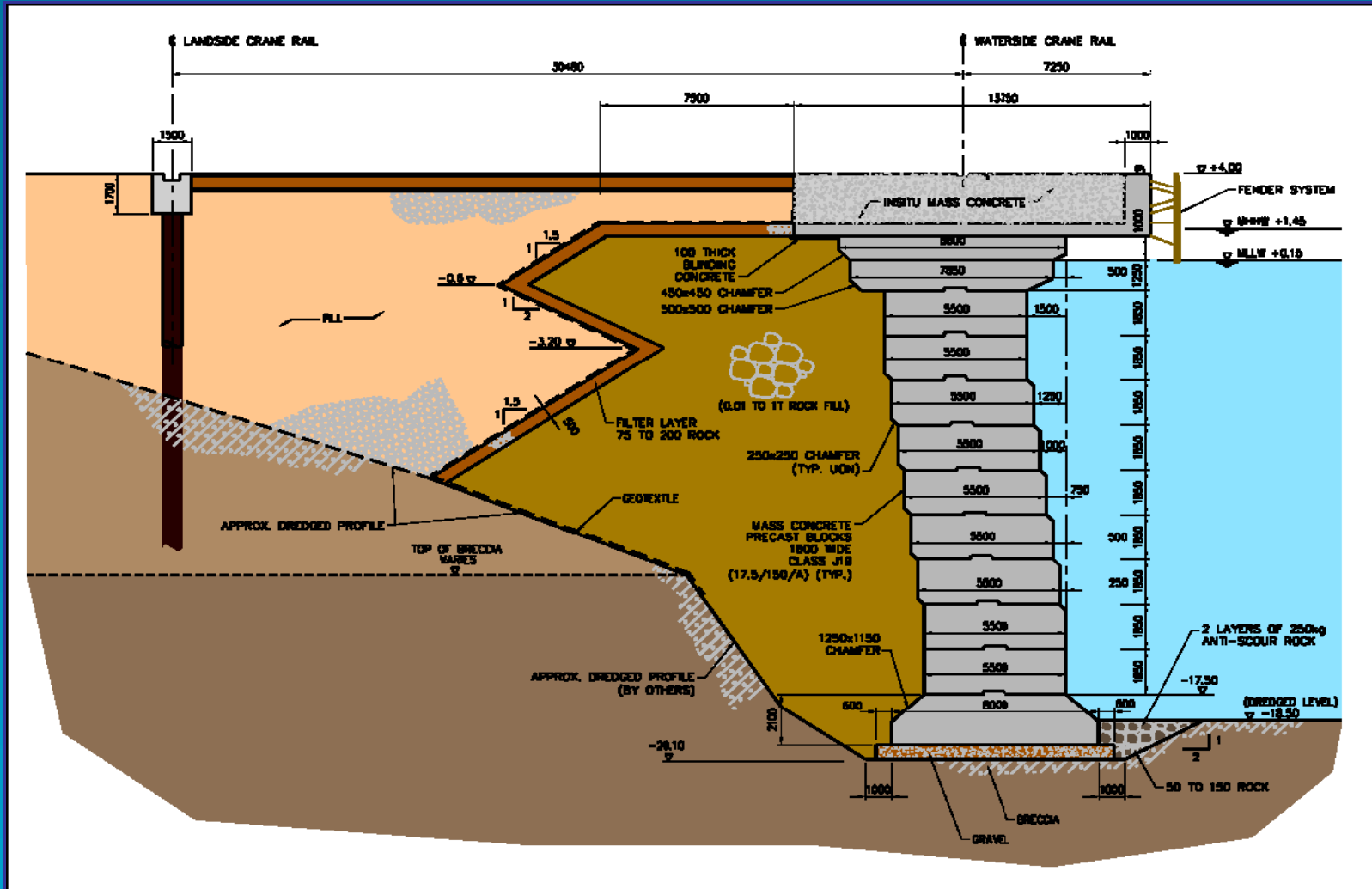








- **Gravity**
- **Pile Supported**

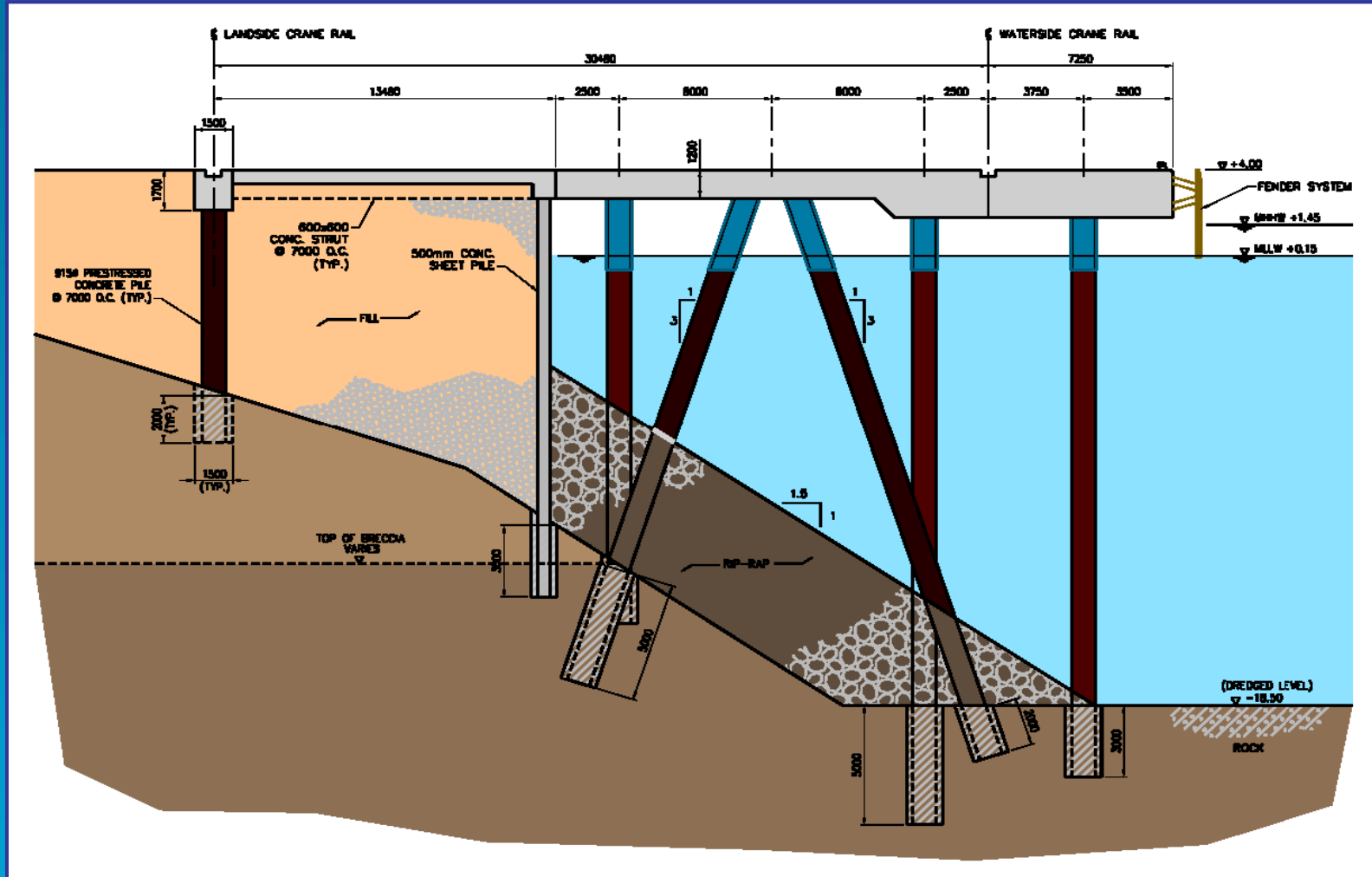


- Cost relatively insensitive to crane loading

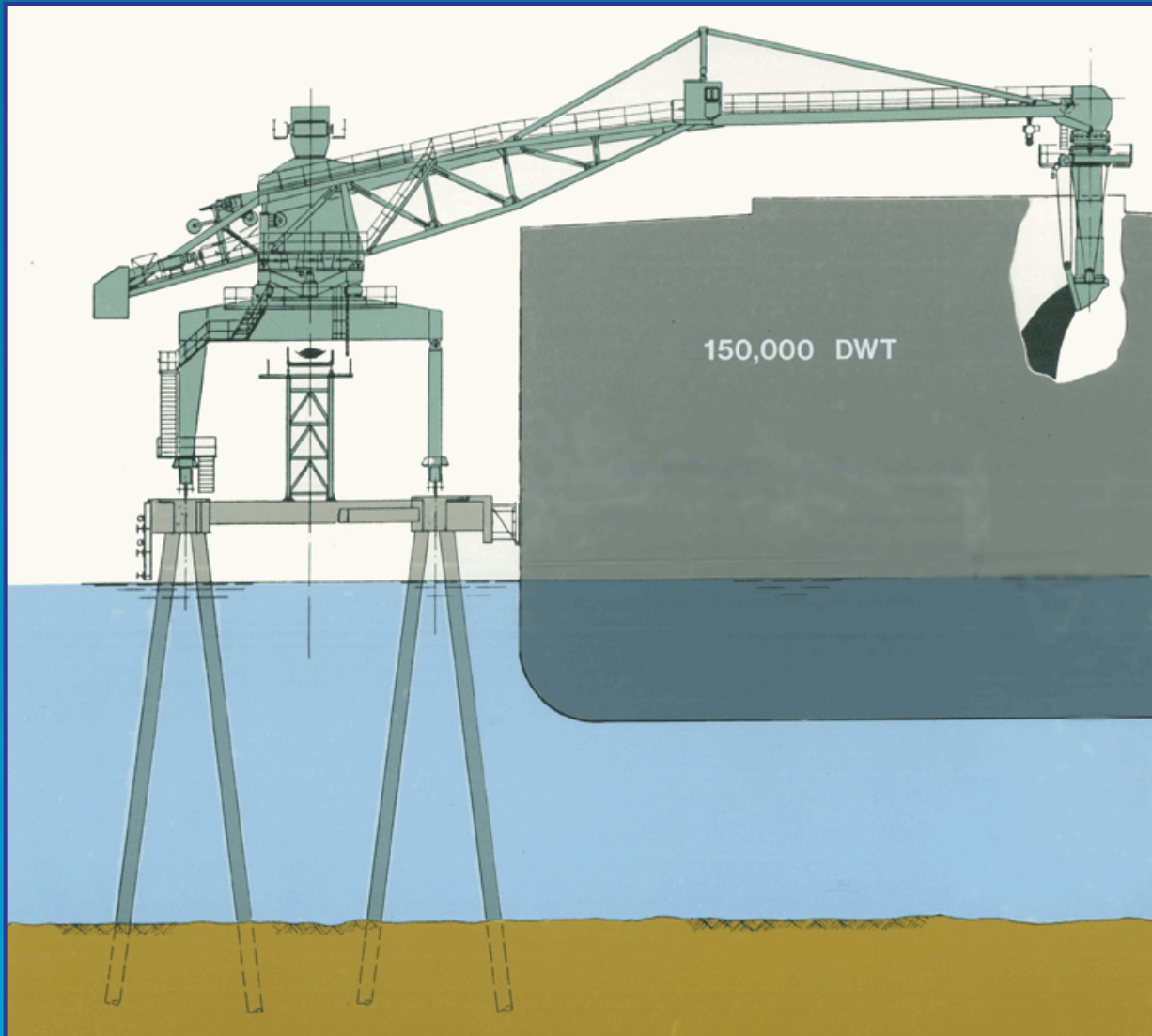




# Typical Pile Supported Wharf Structure



- Cost sensitive to crane loading



- Cost very sensitive to crane loading



- Dimensional
- Loading

- Gage (spacing between rails)
- C – to – c distance between corners
- Number of wheels per corner
- Spacing of wheels



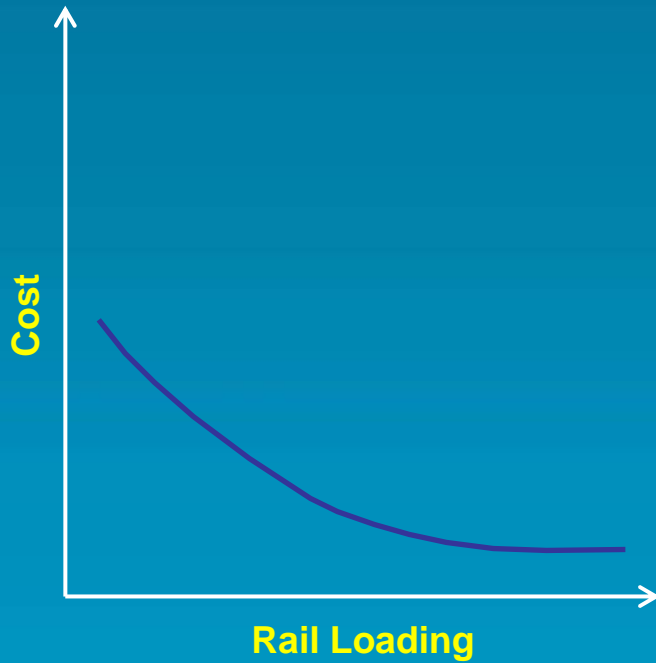
### Maximum Load Per Corner

- **Operating Condition**
  - Water Side Vertical
  - Land Side Vertical
  - Horizontal
- **Extreme Condition**
  - Water Side Vertical
  - Land Side Vertical
  - Horizontal

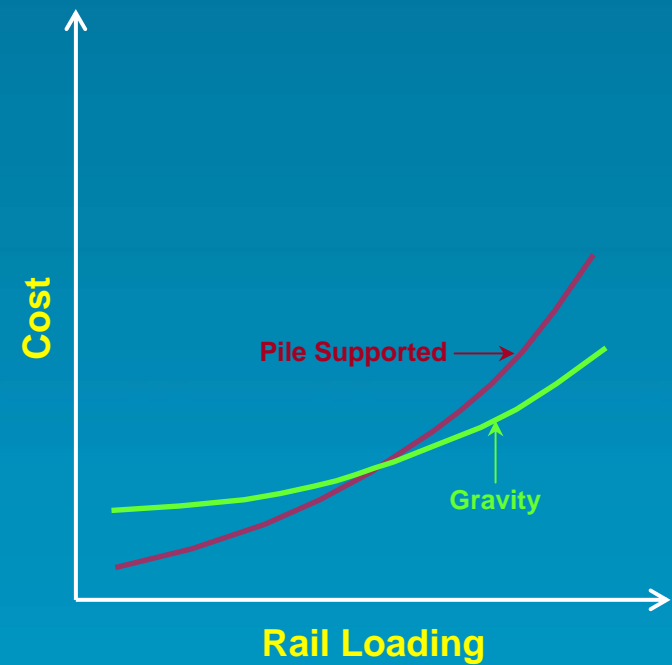


<u>Characteristic</u>	<u>Jebel Ali, Dubai</u>	<u>Doraleh, Djibouti</u>
• Gage (spacing between rails)	42	30.48
• C – to – c distance between corners	14.1	15.5
• Number of wheels per corner	8	8
• Spacing of wheels	1.3	1.2

Load	Maximum Load Per Corner	
	<u>Jebel Ali, Dubai</u>	<u>Doraleh, Djibouti</u>
<ul style="list-style-type: none"> <li>• <b>Operating Condition</b> <ul style="list-style-type: none"> <li>– Water Side Vertical, kN</li> <li>– Land Side Vertical, kN</li> <li>– Horizontal, kN</li> </ul> </li> </ul>	<p><b>11,200</b></p> <p><b>9,600</b></p> <p><b>800</b></p>	<p><b>8,400</b></p> <p><b>6,800</b></p> <p><b>400</b></p>
<ul style="list-style-type: none"> <li>• <b>Extreme Condition</b> <ul style="list-style-type: none"> <li>– Water Side Vertical, kN</li> <li>– Land Side Vertical, kN</li> <li>– Horizontal, kN</li> </ul> </li> </ul>	<p><b>12,000</b></p> <p><b>12,000</b></p> <p><b>2,100</b></p>	<p><b>10,400</b></p> <p><b>8,960</b></p> <p><b>840</b></p>



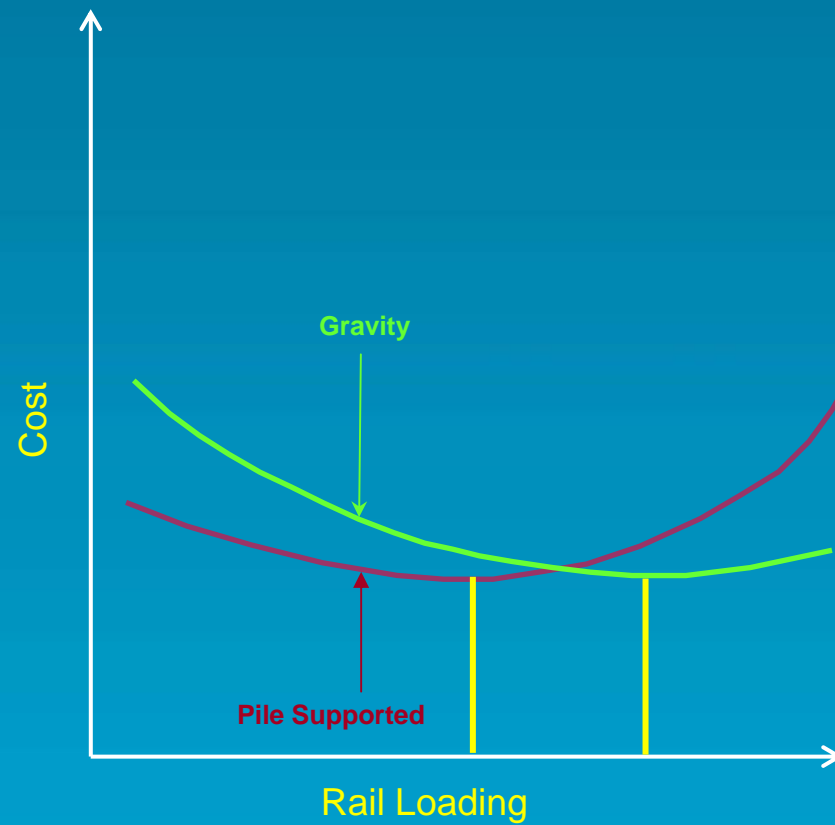
CRANE COST\*



WHARF COST

\* For given crane performance requirements

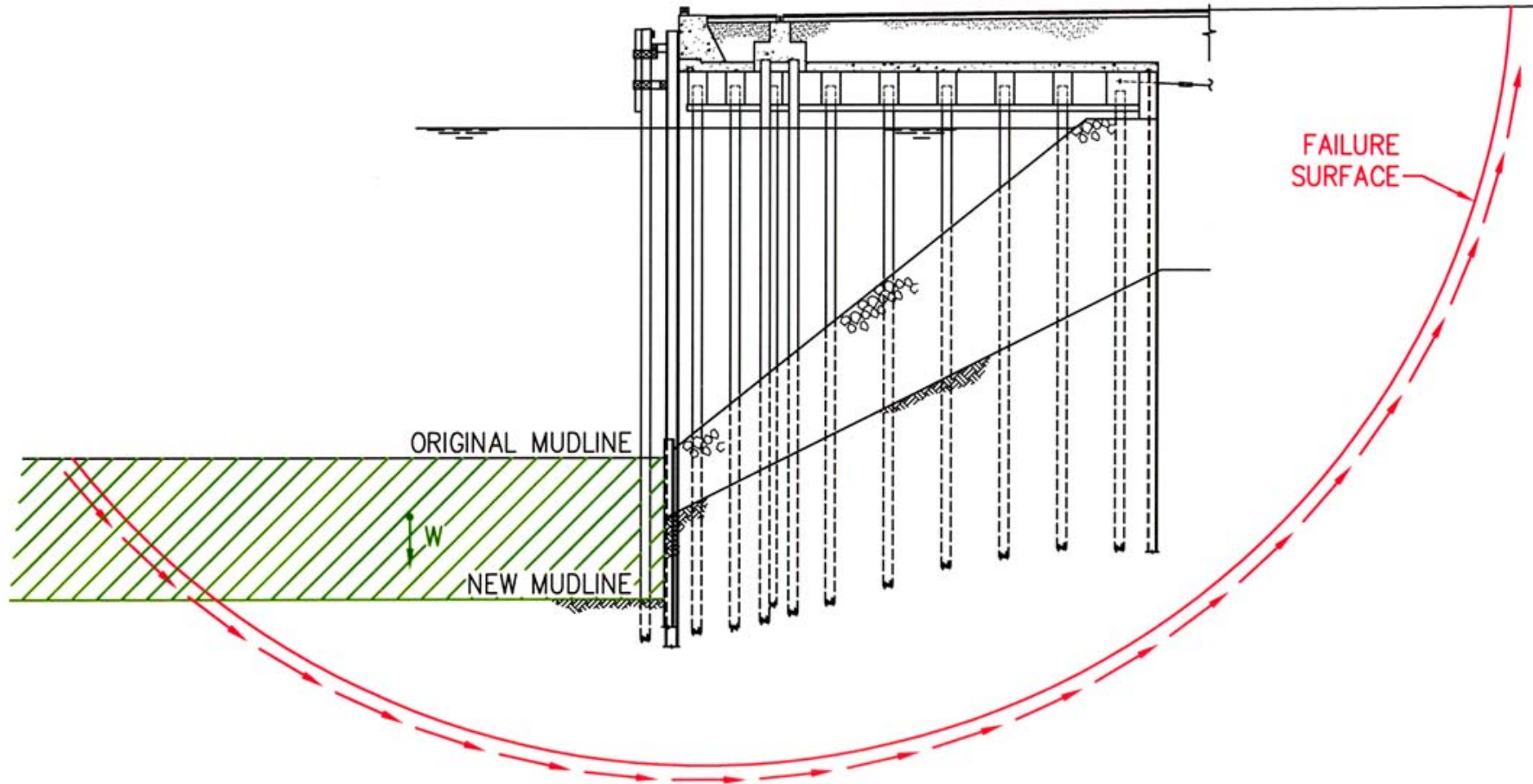


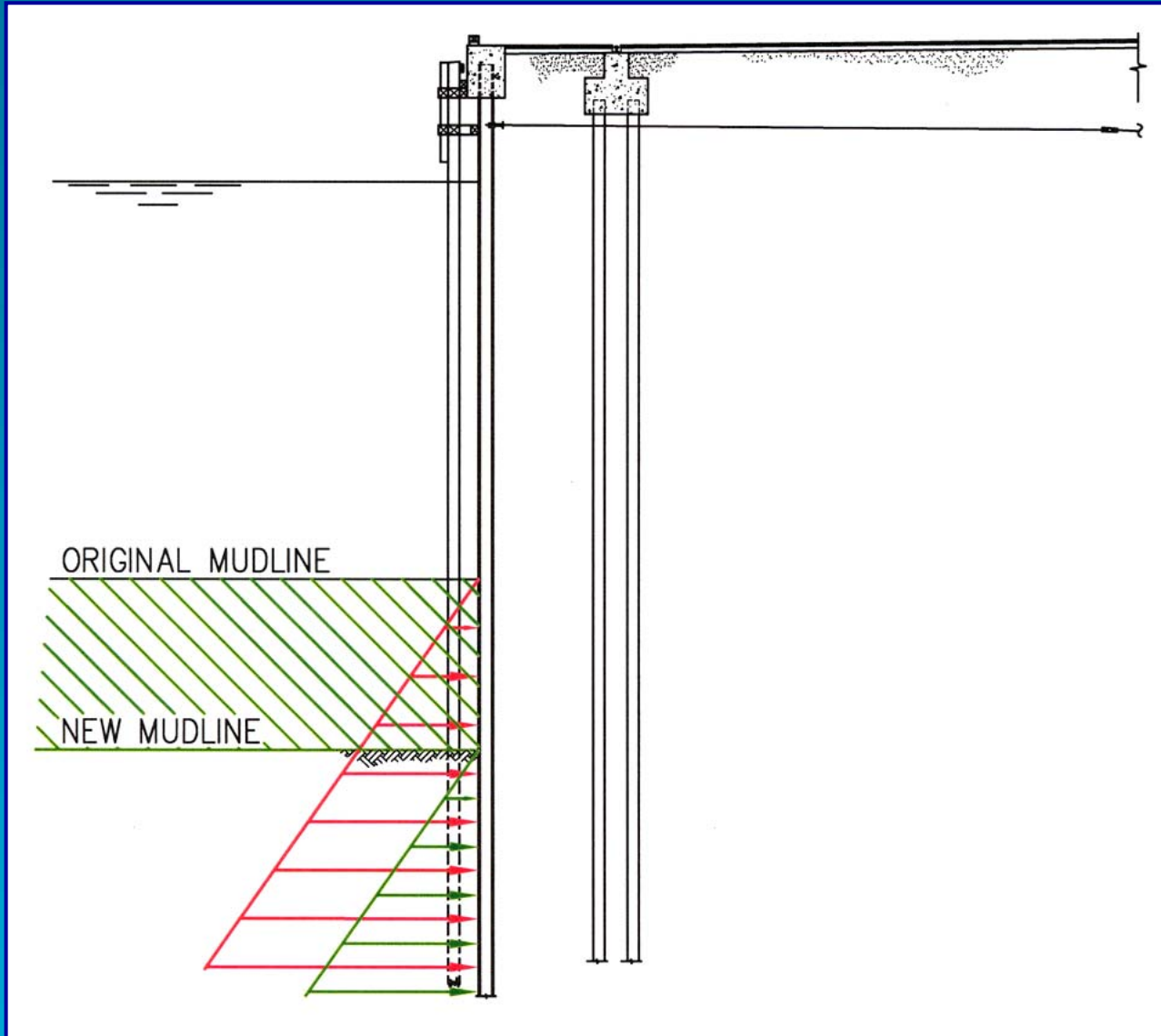


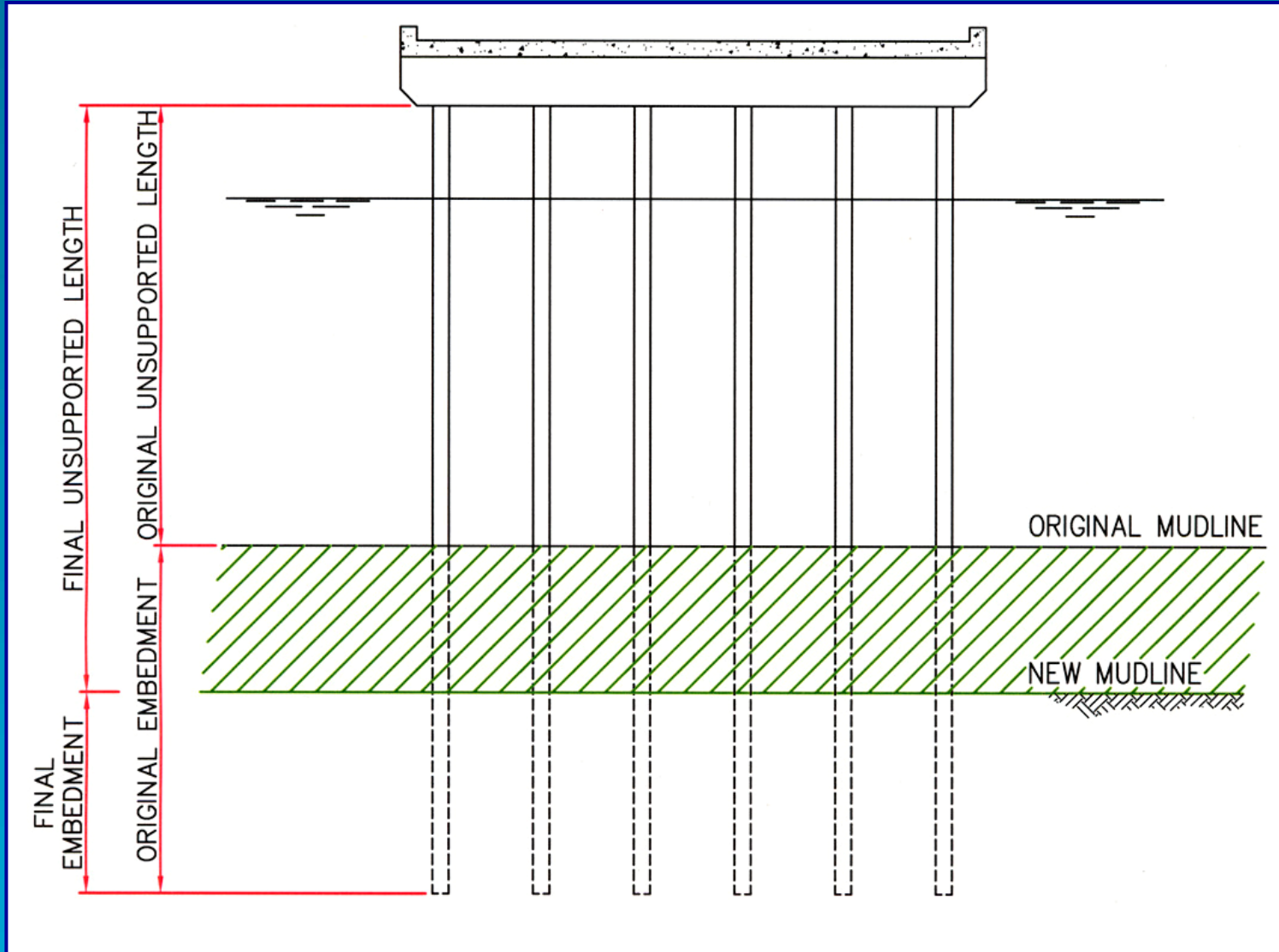
- **Deepening issues**
- **Crane load issues**
- **Fender system**
- **Mooring system**
- **New code requirements**

- **Reduces Global Stability**
- **Reduces Local Stability**
- **Reduces Pile Embedment Capacity**
- **Reduces Pile Structural Capacity**









- Increased reach
- Increased lifting capacity
- Increased weight

## RESULT

- Increased rail loading
- Increased tie-down forces





- Existing Features

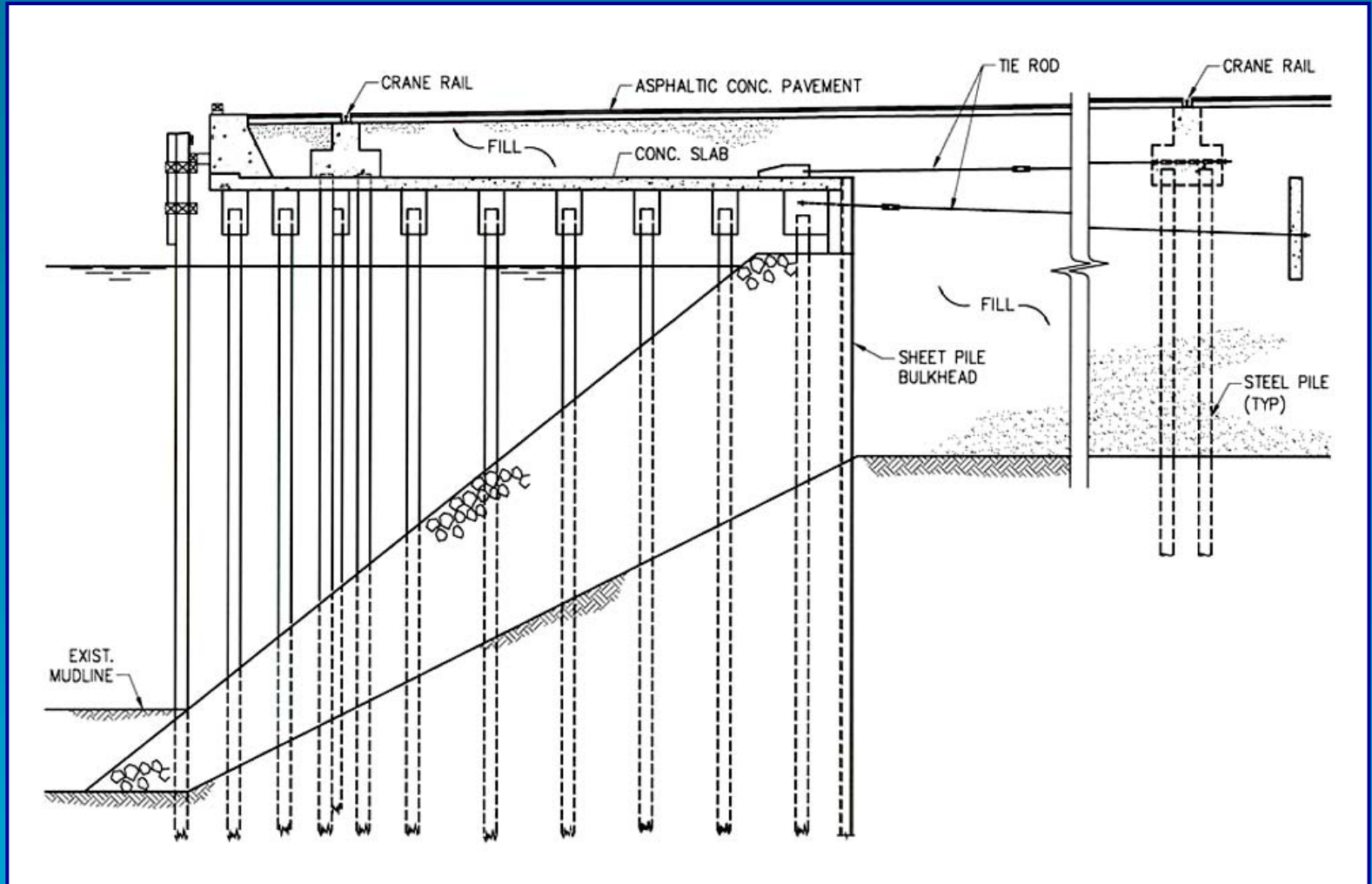
- Constructed ca. 1970
- 40 ft water depth
- 50 ft wide platform
- 30 ton timber piles
- 20 kips per ft rail load
- 100 ft rail gage
- 500 psf deck live load
- 85 ton bollard @ 120 ft
- Minimal timber/rubber fender system
- No seismic criteria

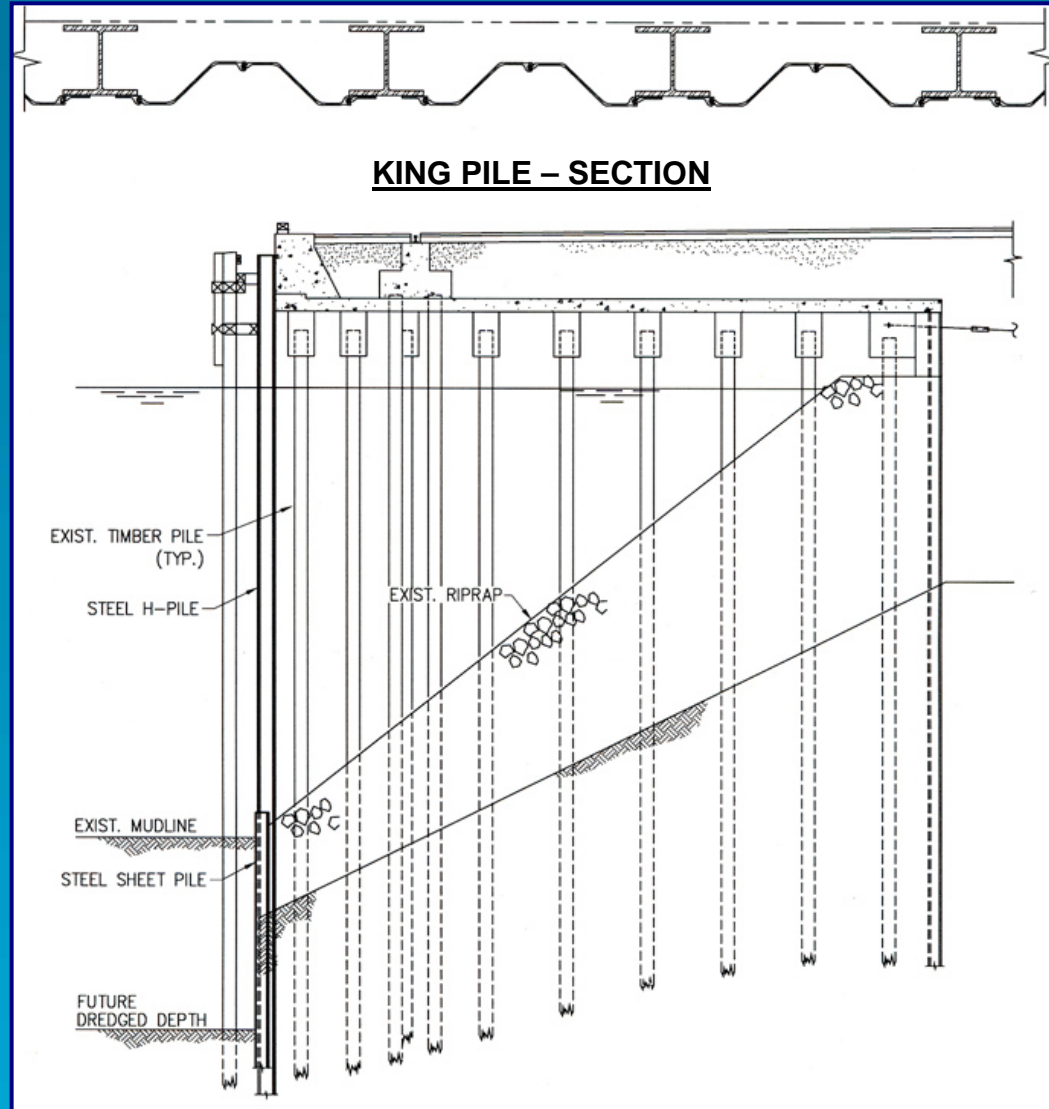
- Upgrading Requirements

- 52 ft water depth
- 250 ton steel piles
- 50 kips per ft rail load
- 165 ton bollards @ 40 ft
- Fender system for Super Post-Panamax vessels
- Seismic zone 2

# APM Port Elizabeth Container Terminal

## Existing Wharf Structure



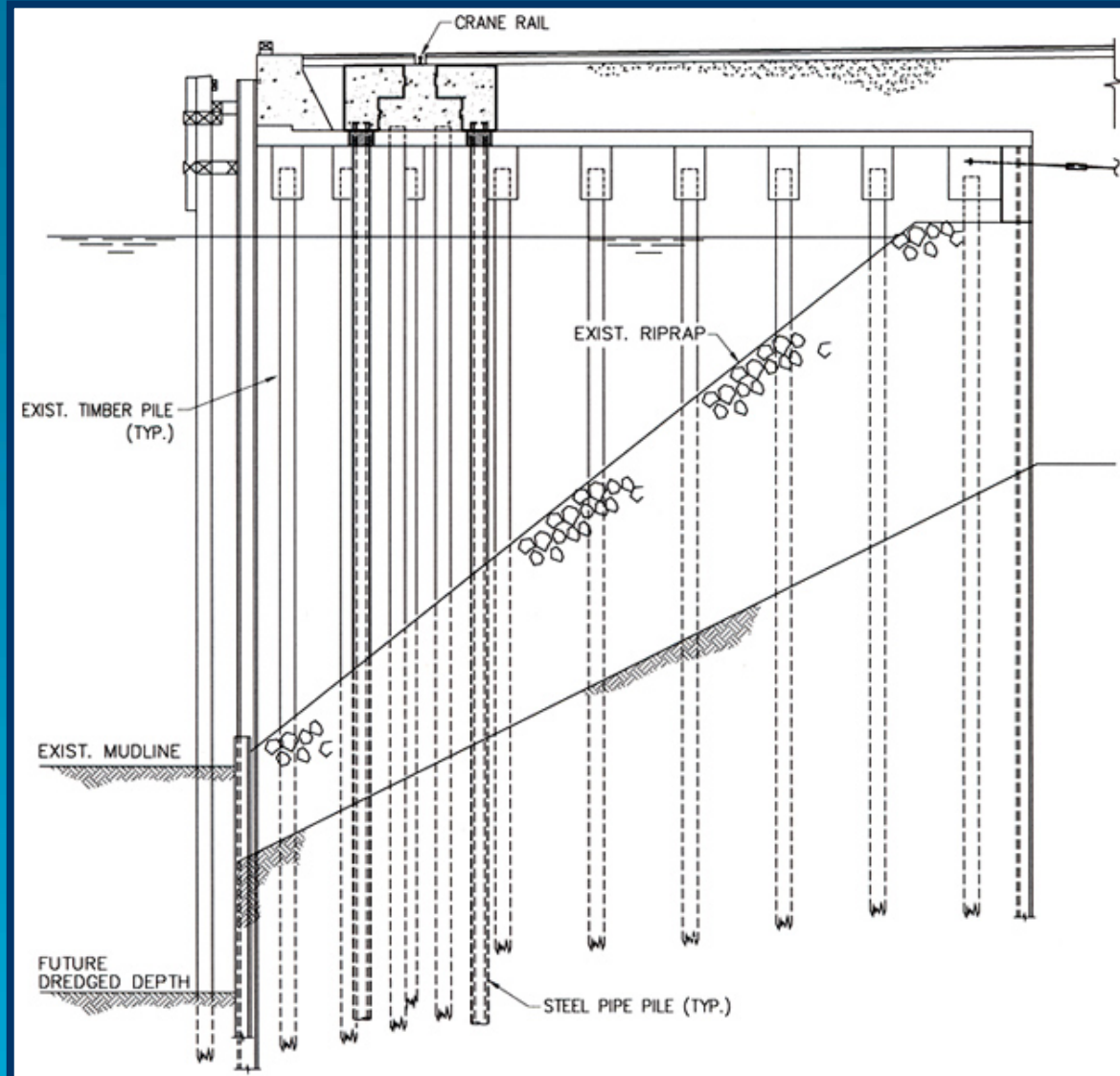


- **Same Location as Existing Rails**
- **Landward of Existing Rails**
- **Seaward of Existing Rails**



# APM Port Elizabeth Container Terminal

## New Rails Same Location as Existing Rails

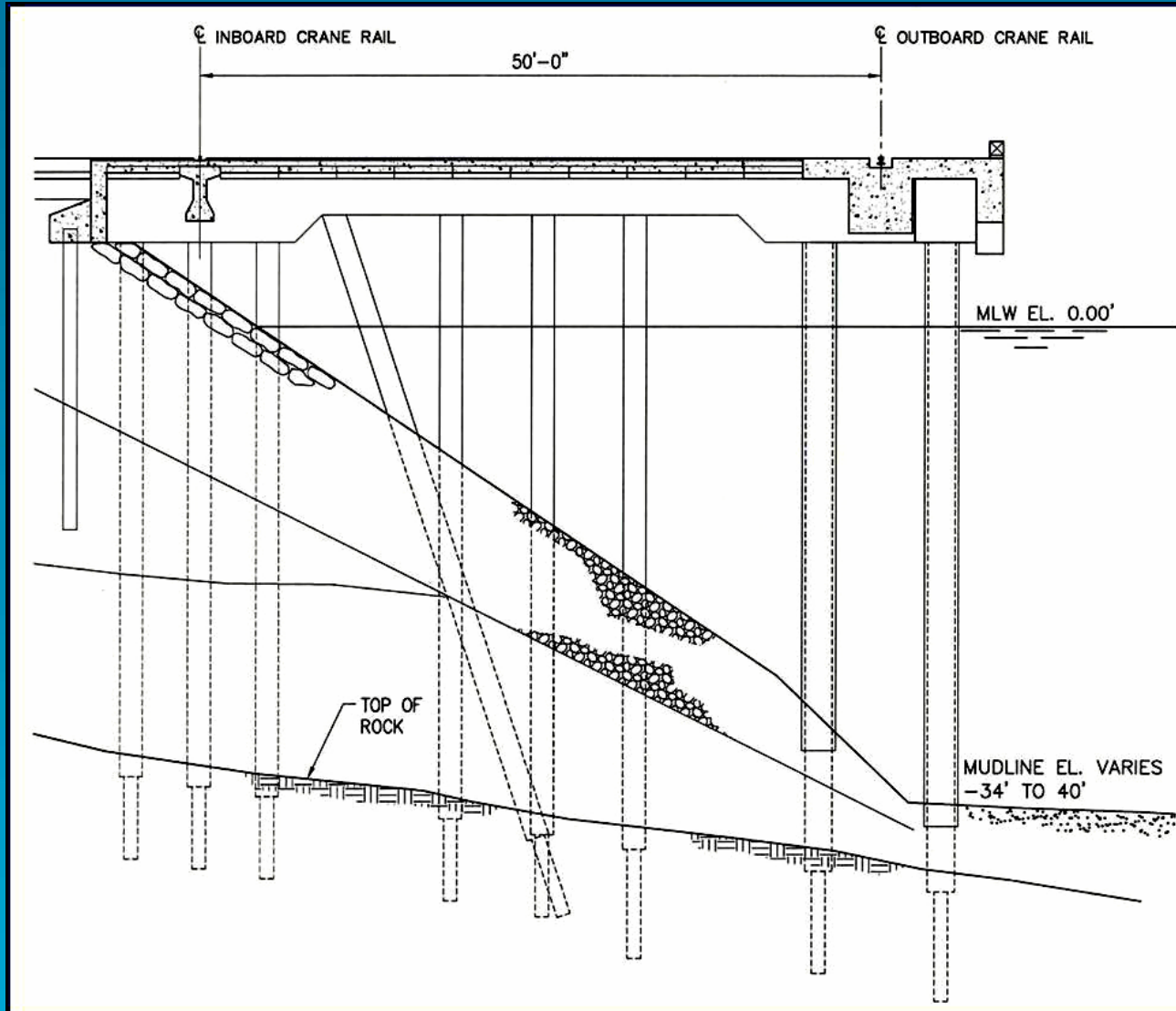




- Existing Features
  - Constructed ca. 1970
  - 35 ft water depth
  - 67 ft wide platform
  - concrete and steel piles
  - 20 kips per ft rail load
  - 50 ft rail gage
  - 500 psf deck live load
  - 85 ton bollard @ 88 ft
  - 15"φ rubber cylinder fender system
  - No seismic criteria
- Upgrading Requirements
  - 52 ft water depth
  - 300 ton steel piles
  - 42 kips per ft rail load
  - 100 ft rail gage
  - 100 ton bollard @ 44 ft
  - Fender system for Super Post-Panamax vessels
  - Seismic zone 2

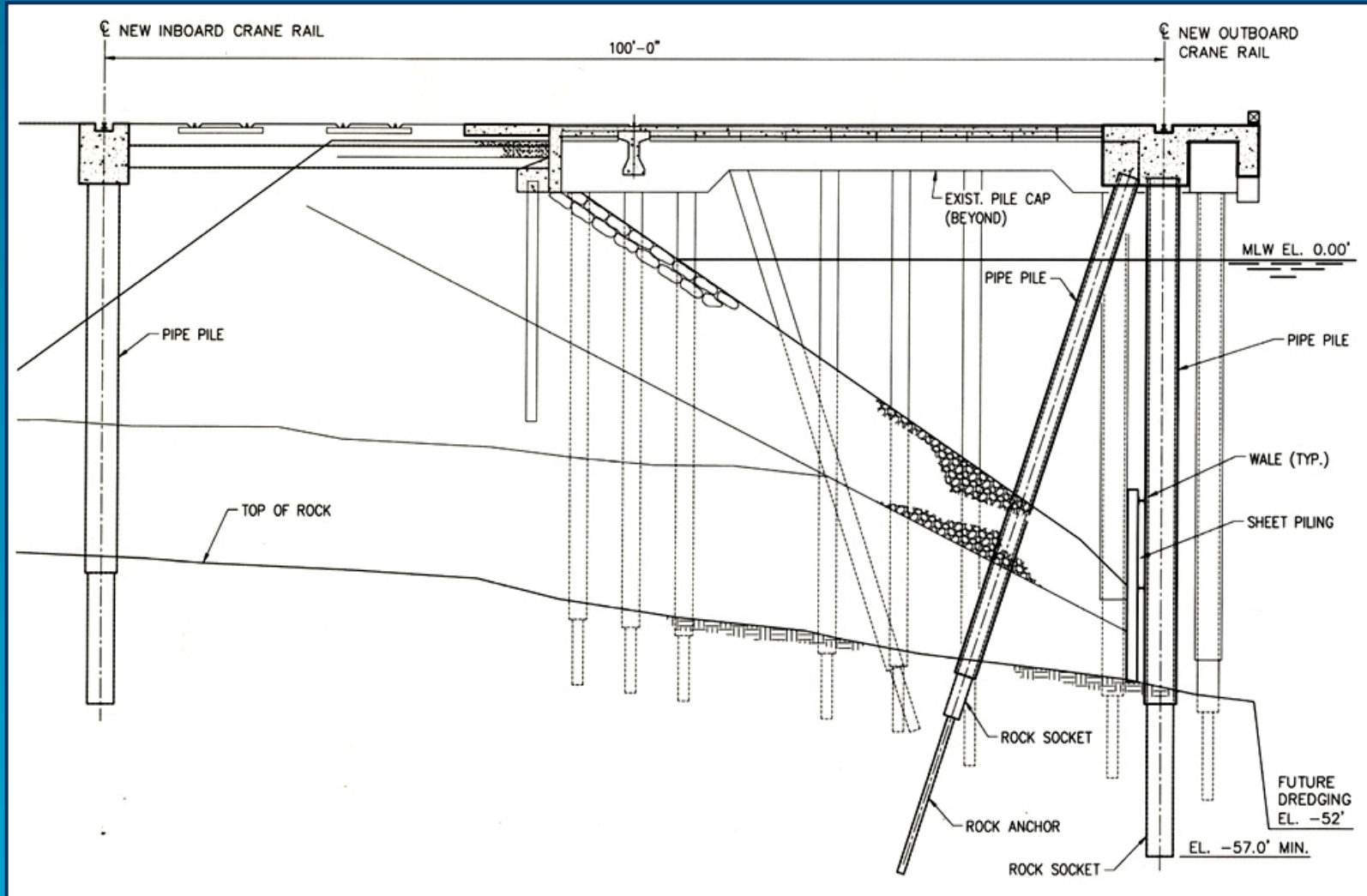
# Howland Hook Container Terminal

## Existing Wharf Structure





# Howland Hook Container Terminal Upgraded Wharf Structure







**This PowerPoint presentation  
can be downloaded from  
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